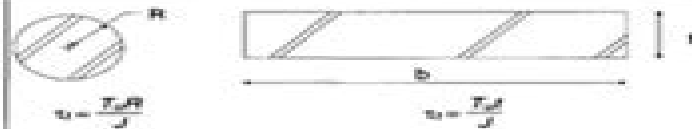


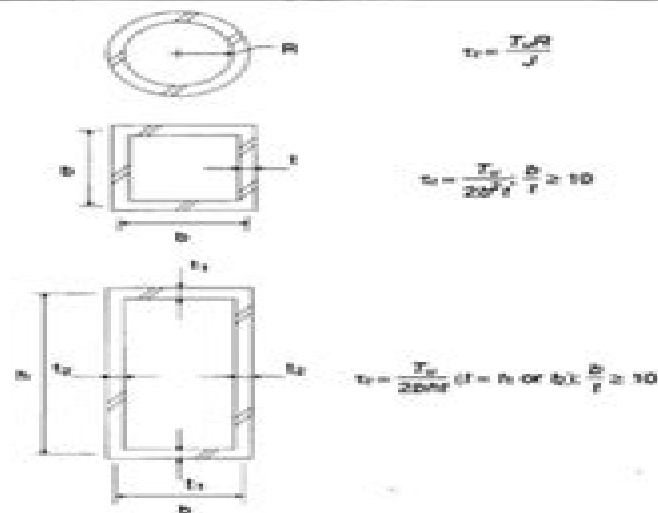
Table 4.1  
Shear Stress Due to St. Venant's Torsion

Solid Cross-Sections



Use appropriate  $J$  from Table 3.1

Closed Cross-Sections



sections tend to warp under torsional loading, this warping is minimized since longitudinal shear prevents relative displacement of adjacent plate elements as illustrated in Figure 4.6.

The analysis and design of thin-walled ( $b/t \geq 10$ ) closed cross-sections for torsion is simplified with the assumption that the torque is absorbed by shear forces that are uniformly distributed over the thickness of the element (Siev, 1966). The general torsional response can be determined from Equation 3.1 with the warping term neglected. For a constant torsional moment  $T$  the shear stress  $\tau_s$  may be calculated as:

$$\tau_s = \frac{T}{2tA_e} \quad (4.4)$$

where

$A_e$  = area enclosed by shape, measured to centerline of thickness of bounding elements as illustrated in Figure 4.7, in.<sup>2</sup>

$t$  = thickness of bounding element, in.

For solid round and flat bars, square, rectangular and round HSS and steel pipe, the torsional shear stress may be calculated using the equations given in Table 4.1. Note that the equation for the hollow circular cross-section in Table 4.1 is not in a form based upon Equation 4.4 and is valid for any wall thickness.

#### 4.5 Elastic Stresses Due to Bending and Axial Load

In addition to the torsional stresses, bending and shear stresses ( $\sigma_x$  and  $\tau_{xy}$  respectively) due to plane bending are normally present in the structural member. These stresses are determined by the following equations:

$$\sigma_x = \frac{M}{S} \quad (4.5)$$

$$\tau_{xy} = \frac{VQ}{It} \quad (4.6)$$

where

$\sigma_x$  = normal stress due to bending about either the  $x$  or  $y$  axis, ksi

$M$  = bending moment about either the  $x$  or  $y$  axis, kip-in.

$S$  = elastic section modulus, in.<sup>3</sup>

$\tau_{xy}$  = shear stress due to applied shear in either  $x$  or  $y$  direction, ksi

$V$  = shear in either  $x$  or  $y$  direction, kips

$Q$  =  $Q_y$  for the maximum shear stress in the flange  
=  $Q_x$  for the maximum shear stress in the web.

$I$  = moment of inertia  $I_x$  or  $I_y$ , in.<sup>4</sup>

$t$  = thickness of element, in.

The value of  $\tau_{xy}$  computed using  $Q_y$  from Appendix A is the theoretical value at the center of the flange. It is within the accuracy of the method presented herein to combine this theoretical value with the torsional shearing stress calculated for the point at the intersection of the web and flange centerlines.

Figure 4.8 illustrates the distribution of these stresses, shown for the case of a moment causing bending about the major axis of the cross-section and shear acting along the minor axis of the cross-section. The stress distribution in the Z-shape is somewhat complicated because the major axis is not parallel to the flanges.

Axial stress  $\sigma_x$  may also be present due to an axial load  $P$ .

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A red circular graphic with a gradient, appearing as a semi-circle or a partial circle, located to the right of the author's name.

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